## RigidLam ${ }^{\circledR}$ LVL Columns

Douglas-fir

AXIAL FACTORED RESISTANCES (LBS) FOR 1.6E RIGIDLAM® LVL COLUMNS

| Effective Column Length (ft.) | Column Size |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $31 / 2^{\prime \prime} \times 31 / 2^{\prime \prime}$ | $31 / 2^{\prime \prime} \times 51 / 2^{\prime \prime}$ | $31 / 2^{\prime \prime} \times 71 / 4$ " | $51 / 4 \prime \times 5^{1 / 2 \prime}$ | $51 / 4 " \times 71 / 4 "$ | 7" $\times 71 / 4$ " |
| 6 | 15,010 | 23,590 | 31,095 | 42,945 | 56,610 | 78,315 |
| 7 | 12,575 | 19,760 | 26,045 | 40,580 | 53,490 | 76,405 |
| 8 | 10,385 | 16,325 | 21,515 | 37,780 | 49,805 | 73,985 |
| 9 | 8,565 | 13,460 | 17,740 | 34,685 | 45,720 | 71,080 |
| 10 | 7,065 | 11,105 | 14,640 | 31,080 | 40,970 | 67,730 |
| 11 | 5,845 | 9,185 | 12,105 | 27,440 | 36,175 | 64,015 |
| 12 | 4,845 | 7,620 | 10,045 | 24,175 | 31,870 | 60,040 |
| 13 | 4,035 | 6,345 | 8,365 | 21,280 | 28,055 | 55,690 |
| 14 | 3,375 | 5,305 | 6,995 | 18,735 | 24,695 | 50,805 |
| 15 | - | - | - | 16,505 | 21,755 | 46,270 |
| 16 | - | - | - | 14,550 | 19,180 | 42,095 |
| 17 | - | - | - | 12,845 | 16,930 | 38,280 |
| 18 | - | - | - | 11,355 | 14,965 | 34,810 |
| 19 | - | - | - | 10,055 | 13,250 | 31,660 |
| 20 | - | - | - | 8,915 | 11,755 | 28,805 |
| 21 | - | - | - | 7,925 | 10,450 | 26,220 |
| 22 | - | - | - | - | - | 23,885 |
| 23 | - | - | - | - | - | 21,770 |
| 24 | - | - | - | - | - | 19,860 |
| 25 | - | - | - | - | - | 18,130 |

## Notes:

1. Column is a single, one-piece member for dry-use applications only.
2. Column is assumed to have adequate bracing in all directions at both ends.
3. Loads are calculated per Section 5.1 of CWC "Wood Design Manual 2010" and CSA O86-14 for simple columns with axial loads only.
4. For side-loaded columns, see the CSA O86-14 provisions or consult with a design professional.
5. Table assumes the worst case of an eccentricity of $1 / 6$ of either column dimension.
6. Table assumes column bearing to be on a steel plate that has been adequately sized for bearing on the material below.
7. When bearing on a $11 / 2^{\prime \prime}$ thick wood plate, axial factored loads (lbs) shall not exceed the following values:

| Column Size | $31 / 2^{\prime \prime} \times 31 / 2^{\prime \prime}$ | $31 / 2^{\prime \prime} \times 5^{1 / 2}{ }^{\prime \prime}$ | $31 / 2 " \times 71 / 4$ " | $51 / 4 \prime \times 51 / 2^{\prime \prime}$ | $51 / 41 \times 71 / 4^{\prime \prime}$ | 7" $\times 71 / 4$ " |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D Fir-L plate | $11,439 \mathrm{lbs}$ | 17,975 lbs | 23,695 lbs | 26,963 lbs | 35,542 lbs | 47,389 lbs |
| Hem-Fir plate | 7,517 lbs | $11,812 \mathrm{lbs}$ | 15,571 lbs | 17,719 lbs | $23,356 \mathrm{lbs}$ | 31,142 lbs |
| Spruce-Pine-Fir plate | 8,661 lbs | 13,610 lbs | 17,940 lbs | 20,415 lbs | $26,910 \mathrm{lbs}$ | 35,880 lbs |
| Northern plate | 5,720 lbs | 8,989 lbs | 11,849 lbs | 13,483 lbs | $17,773 \mathrm{lbs}$ | 23,697 lbs |

### 1.6E RigidLam ${ }^{\circledR}$ LVL Specified Strengths ${ }^{(1)}$

True Modulus of Elasticity
Bending Edgewise
Bending Flatwise
Compression Parallel to Grain
$\mathrm{E}=1,600,000 \mathrm{psi}$
$\mathrm{Fb}=4,158 \mathrm{psi}^{(2)}$
$\mathrm{Fb}=4,064 \mathrm{psi}{ }^{(3)}$
$\mathrm{Fc}=3,112 \mathrm{psi}$
(1) These specified strengths are for standard term load duration and apply to dry service conditions.
(2) The tabulated values are based on a reference depth of 12 inches. For other depths, when loaded edgewise, the allowable bending stress ( Fb ) shall be modified by a depth factor, Kzb $=(12 / d) \wedge 1 / 8$ for Douglas-fir LVL (Mill \#1055), where $d$ is the LVL depth in inches. For depths less than $31 / 2$ inches, multiply the value by 1.17 for DF LVL.
(3) Tabulated Fb Tflat values are based on a thickness of $13 / 4^{\prime \prime}$. For other thicknesses, when loaded flatwise, multiply flat by $(1.75 / \mathrm{t})^{\wedge 1} / 5$, where $t$ is the LVL thickness in inches. For thicknesses less than $13 / 4$ ", use the tabulates value.

## RigidLam ${ }^{\circledR}$ LVL Columns

Douglas-fir

AXIAL FACTORED RESISTANCES (LBS) FOR 2.1E RIGIDLAM® LVL COLUMNS

| Effective Column Length (ft.) | Column Size |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $31 / 2^{\prime \prime} \times 31 / 2^{\prime \prime}$ | $31 / 2^{\prime \prime} \times 51 / 2^{\prime \prime}$ | $31 / 2^{\prime \prime} \times 71 / 4^{\prime \prime}$ | $51 / 4 " \times 51 / 2^{\prime \prime}$ | $51 / 4 " \times 71 / 4$ " | 7" $\times 71 / 4$ " |
| 6 | 20,885 | 32,820 | 43,265 | 60,020 | 79,115 | 109,105 |
| 7 | 17,230 | 27,075 | 35,690 | 56,640 | 74,660 | 106,420 |
| 8 | 14,150 | 22,235 | 29,310 | 52,640 | 69,390 | 103,020 |
| 9 | 11,620 | 18,265 | 24,075 | 48,205 | 63,545 | 98,910 |
| 10 | 9,565 | 15,035 | 19,820 | 42,615 | 56,175 | 94,165 |
| 11 | 7,895 | 12,410 | 16,360 | 37,455 | 49,375 | 88,890 |
| 12 | 6,540 | 10,280 | 13,550 | 32,885 | 43,350 | 83,240 |
| 13 | 5,440 | 8,550 | 11,270 | 28,875 | 38,065 | 76,350 |
| 14 | 4,545 | 7,145 | 9,415 | 25,375 | 33,445 | 69,405 |
| 15 | - | - | - | 22,320 | 29,420 | 63,020 |
| 16 | - | - | - | 19,655 | 25,910 | 57,200 |
| 17 | - | - | - | 17,335 | 22,850 | 51,920 |
| 18 | - | - | - | 15,310 | 20,180 | 47,140 |
| 19 | - | - | - | 13,545 | 17,855 | 42,825 |
| 20 | - | - | - | 12,005 | 15,825 | 38,925 |
| 21 | - | - | - | 10,665 | 14,060 | 35,405 |
| 22 | - | - | - | - | - | 32,225 |
| 23 | - | - | - | - | - | 29,355 |
| 24 | - | - | - | - | - | 26,765 |
| 25 | - | - | - | - | - | 24,425 |

## Notes:

1. Column is a single, one-piece member for dry-use applications only.
2. Column is assumed to have adequate bracing in all directions at both ends.
3. Loads are calculated per Section 5.1 of CWC "Wood Design Manual 2010" and CSA O86-14 for simple columns with axial loads only.
4. For side-loaded columns, see the CSA O86-14 provisions or consult with a design professional.
5. Table assumes the worst case of an eccentricity of $1 / 6$ of either column dimension.
6. Table assumes column bearing to be on a steel plate that has been adequately sized for bearing on the material below.
7. When bearing on a $11 / 2^{\prime \prime}$ thick wood plate, axial factored loads (lbs) shall not exceed the following values:

| Column Size | $31 / 2^{\prime \prime} \times 31 / 2^{\prime \prime}$ | $31 / 2{ }^{\prime \prime} \times 51 / 2^{\prime \prime}$ | $31 / 2$ " $\times 71 / 4$ " | $51 / 4 \prime \times 51 / 2^{\prime \prime}$ | 51/4" $\times 71 / 4$ " | 7" $\times 71 / 4$ " |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D Fir-L plate | 11,439 lbs | $17,975 \mathrm{lbs}$ | 23,695 lbs | 26,963 lbs | 35,542 lbs | 47,389 lbs |
| Hem-Fir plate | 7,517 lbs | 11,812 lbs | 15,571 lbs | $17,719 \mathrm{lbs}$ | 23,356 lbs | 31,142 lbs |
| Spruce-Pine-Fir plate | 8,661 lbs | 13,610 lbs | 17,940 lbs | 20,415 lbs | 26,910 lbs | 35,880 lbs |
| Northern plate | 5,720 lbs | 8,989 lbs | 11,849 lbs | 13,483 lbs | 17,773 lbs | 23,697 lbs |

### 2.1E RigidLam ${ }^{\circledR}$ LVL Specified Strengths ${ }^{(1)}$

True Modulus of Elasticity Bending Edgewise Bending Flatwise Compression Parallel to Grain
$\mathrm{E}=2,100,000 \mathrm{psi}$
$\mathrm{Fb}=5,729 \mathrm{psi}^{(2)}$
$\mathrm{Fb}=5,013 \mathrm{psi}{ }^{(3)}$
$\mathrm{Fc}=4,788 \mathrm{psi}$
(1) These specified strengths are for standard term load duration and apply to dry service conditions.
(2) The tabulated values are based on a reference depth of 12 inches. For other depths, when loaded edgewise, the allowable bending stress ( Fb ) shall be modified by a depth factor, Kzb $=(12 / d)^{\wedge 1} / 8$ for Douglas-fir LVL (Mill \#1055), where d is the LVL depth in inches. For depths less than $31 / 2$ inches, multiply the value by 1.17 for DF LVL.
(3) Tabulated Fb Tflat values are based on a thickness of $13 / 4^{\prime \prime}$. For other thicknesses, when loaded flatwise, multiply flat by $(1.75 / \mathrm{t})^{\wedge 1} / 5$, where t is the LVL thickness in inches. For thicknesses less than $13 / 4$ ", use the tabulates value.

## RigidLam ${ }^{\circledR}$ LVL Columns

Douglas-fir

AXIAL FACTORED RESISTANCES (LBS) FOR 2.3E RIGIDLAM® LVL COLUMNS

| Effective Column Length (ft.) | Column Size |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $31 / 2^{\prime \prime} \times 31 / 2^{\prime \prime}$ | $31 / 2^{\prime \prime} \times 51 / 2^{\prime \prime}$ | $31 / 2^{\prime \prime} \times 71 / 4^{\prime \prime}$ | $51 / 4 " \times 51 / 2^{\prime \prime}$ | $51 / 4 " \times 71 / 4 "$ | 7" $\times 71 / 4$ " |
| 6 | 22,300 | 35,040 | 46,195 | 63,775 | 84,065 | 116,115 |
| 7 | 18,585 | 29,205 | 38,495 | 60,245 | 79,415 | 113,290 |
| 8 | 15,325 | 24,080 | 31,745 | 56,075 | 73,915 | 109,705 |
| 9 | 12,620 | 19,835 | 26,145 | 51,450 | 67,820 | 105,395 |
| 10 | 10,405 | 16,355 | 21,560 | 45,940 | 60,560 | 100,415 |
| 11 | 8,605 | 13,520 | 17,820 | 40,505 | 53,390 | 94,895 |
| 12 | 7,135 | 11,210 | 14,780 | 35,645 | 46,990 | 88,975 |
| 13 | 5,940 | 9,335 | 12,305 | 31,355 | 41,330 | 82,300 |
| 14 | 4,965 | 7,805 | 10,290 | 27,590 | 36,365 | 74,995 |
| 15 | - | - | - | 24,290 | 32,020 | 68,235 |
| 16 | - | - | - | 21,415 | 28,225 | 62,035 |
| 17 | - | - | - | 18,900 | 24,910 | 56,385 |
| 18 | - | - | - | 16,705 | 22,020 | 51,250 |
| 19 | - | - | - | 14,785 | 19,490 | 46,595 |
| 20 | - | - | - | 13,115 | 17,285 | 42,385 |
| 21 | - | - | - | 11,655 | 15,365 | 38,575 |
| 22 | - | - | - | - | - | 35,135 |
| 23 | - | - | - | - | - | 32,020 |
| 24 | - | - | - | - | - | 29,205 |
| 25 | - | - | - | - | - | 26,665 |

## Notes:

1. Column is a single, one-piece member for dry-use applications only.
2. Column is assumed to have adequate bracing in all directions at both ends.
3. Loads are calculated per Section 5.1 of CWC "Wood Design Manual 2010" and CSA O86-14 for simple columns with axial loads only.
4. For side-loaded columns, see the CSA O86-14 provisions or consult with a design professional.
5. Table assumes the worst case of an eccentricity of $1 / 6$ of either column dimension.
6. Table assumes column bearing to be on a steel plate that has been adequately sized for bearing on the material below.
7. When bearing on a $11 / 2^{\prime \prime}$ thick wood plate, axial factored loads (lbs) shall not exceed the following values:

| Column Size | $31 / 2^{\prime \prime} \times 31 / 2^{\prime \prime}$ | $31 / 2{ }^{\prime \prime} \times 51 / 2^{\prime \prime}$ | $31 / 2$ " $\times 71 / 4$ " | $51 / 4 \prime \times 51 / 2^{\prime \prime}$ | 51/4" $\times 71 / 4$ " | 7" $\times 71 / 4$ " |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D Fir-L plate | 11,439 lbs | $17,975 \mathrm{lbs}$ | 23,695 lbs | 26,963 lbs | 35,542 lbs | 47,389 lbs |
| Hem-Fir plate | 7,517 lbs | 11,812 lbs | 15,571 lbs | $17,719 \mathrm{lbs}$ | 23,356 lbs | 31,142 lbs |
| Spruce-Pine-Fir plate | 8,661 lbs | 13,610 lbs | 17,940 lbs | 20,415 lbs | 26,910 lbs | 35,880 lbs |
| Northern plate | 5,720 lbs | 8,989 lbs | 11,849 lbs | 13,483 lbs | 17,773 lbs | 23,697 lbs |

### 2.3E RigidLam ${ }^{\circledR}$ LVL Specified Strengths ${ }^{(1)}$

True Modulus of Elasticity
Bending Edgewise
Bending Flatwise
Compression Parallel to Grain
$\mathrm{E}=2,300,000 \mathrm{psi}$
$\mathrm{Fb}=5,729 \mathrm{psi}^{(2)}$
$\mathrm{Fb}=5,729 \mathrm{psi}^{(3)}$
$\mathrm{Fc}=4,788 \mathrm{psi}$
(1) These specified strengths are for standard term load duration and apply to dry service conditions.
(2) The tabulated values are based on a reference depth of 12 inches. For other depths, when loaded edgewise, the allowable bending stress ( Fb ) shall be modified by a depth factor, Kzb $=(12 / d) \wedge 1 / 8$ for Douglas-fir LVL (Mill \#1055), where $d$ is the LVL depth in inches. For depths less than $31 / 2$ inches, multiply the value by 1.17 for DF LVL.
(3) Tabulated Fb Tflat values are based on a thickness of $13 / 4$ ". For other thicknesses, when loaded flatwise, multiply flat by $(1.75 / t)^{\wedge 1} / 5$, where $t$ is the LVL thickness in inches. For thicknesses less than $13 / 4$ ", use the tabulates value.

